



Integrated Research on Earthquake Disaster of High Dams, Underground Structures and Large-scale Cavern Groups

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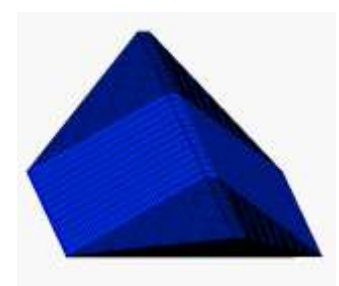
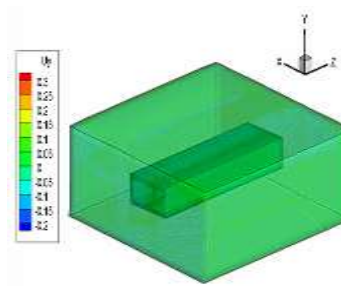
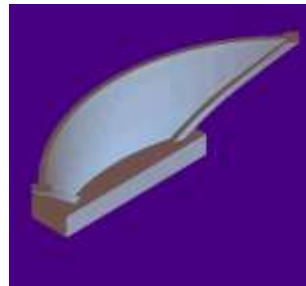
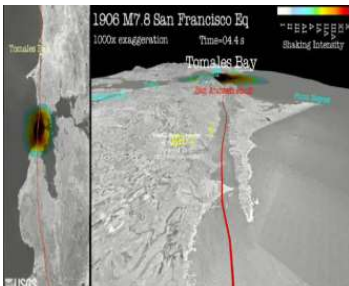
2016.11.19

Contents

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- II. Overall research methodology**
- III. Important progress and innovation**
- IV. Summary and prospect**

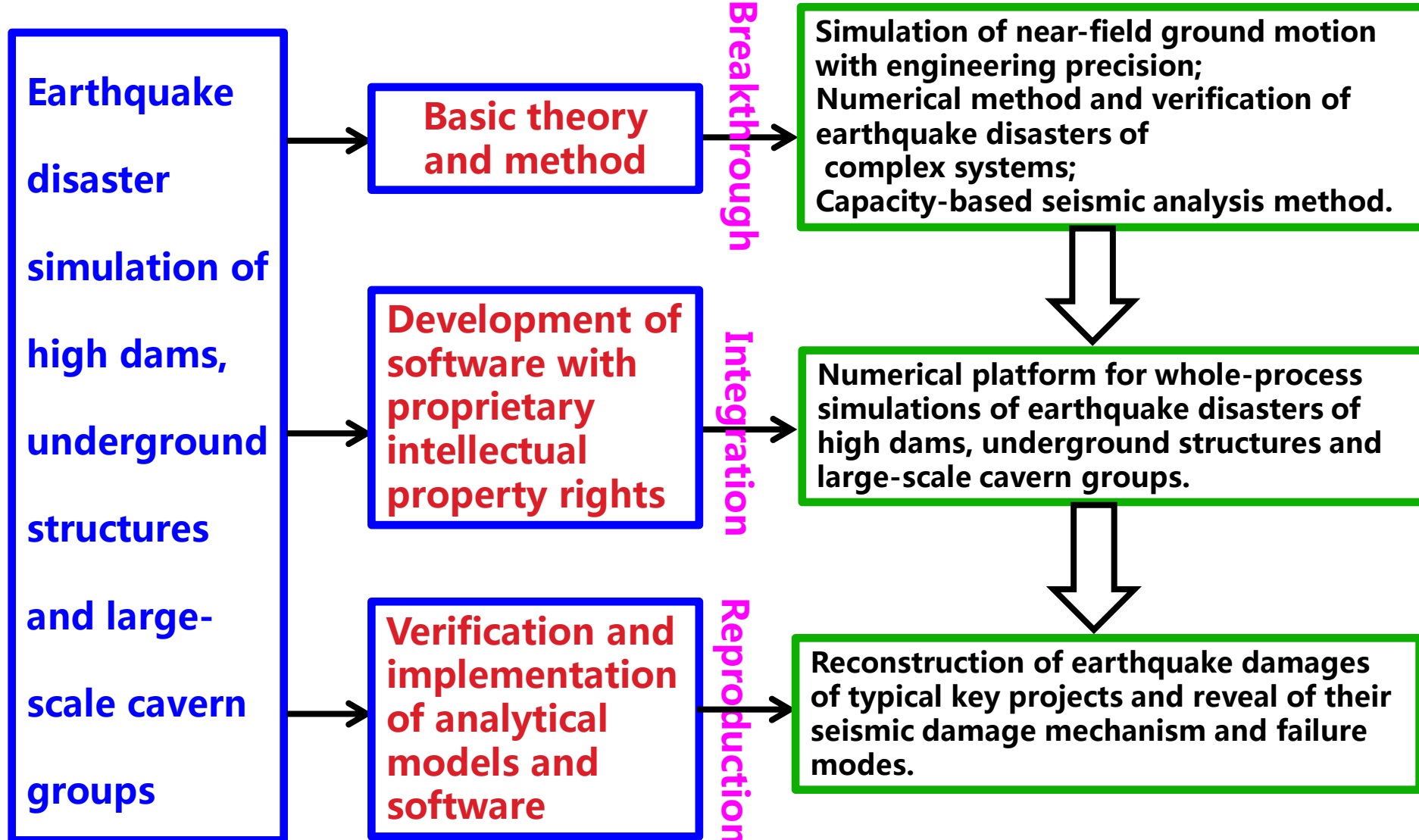
1. Objectives

- 1) Simulation of wideband strong ground motions with both high and low frequency contents, and reproduction of seismic motions fields of the Wenchuan earthquake;
- 2) Whole-process simulations of earthquake disasters of high dams, underground structures and large-scale cavern groups;
- 3) Reconstruction of earthquake damages of typical key projects and reveal of their seismic damage mechanism and failure modes.



I. Objectives and tasks

2. Main research contents



3. Key scientific problems

- 1) Investigation of characteristics of near-field ground motions and simulations of broadband earthquake motions;**
- 2) Mechanical behaviors and constitutive models of rock and soil materials, and contact interfaces;**
- 3) Seismic failure mechanisms of high dams, underground structures and large-scale cavern groups;**
- 4) Development of efficient calculation techniques in software for seismic simulations of earthquake disasters.**

Contents

I. Objectives and tasks

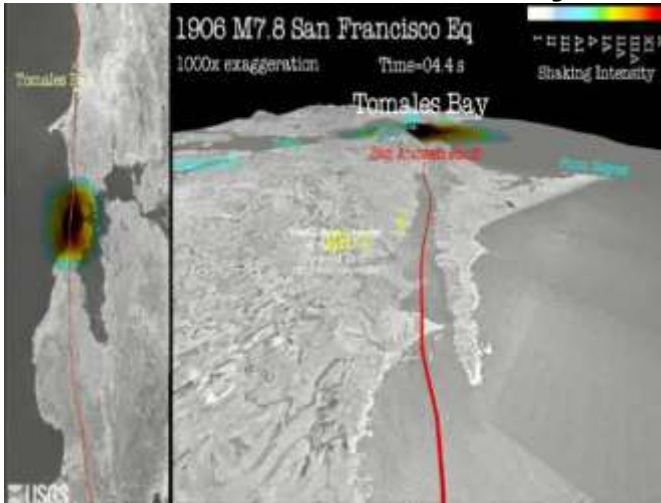
II. Overall research methodology

III. Important progress and innovation

IV. Summary and prospect

II. Overall research methodology

Seismic source rupture + Propagation of energy in a semi infinite medium →
Dynamic response of Structures



concrete arch dam



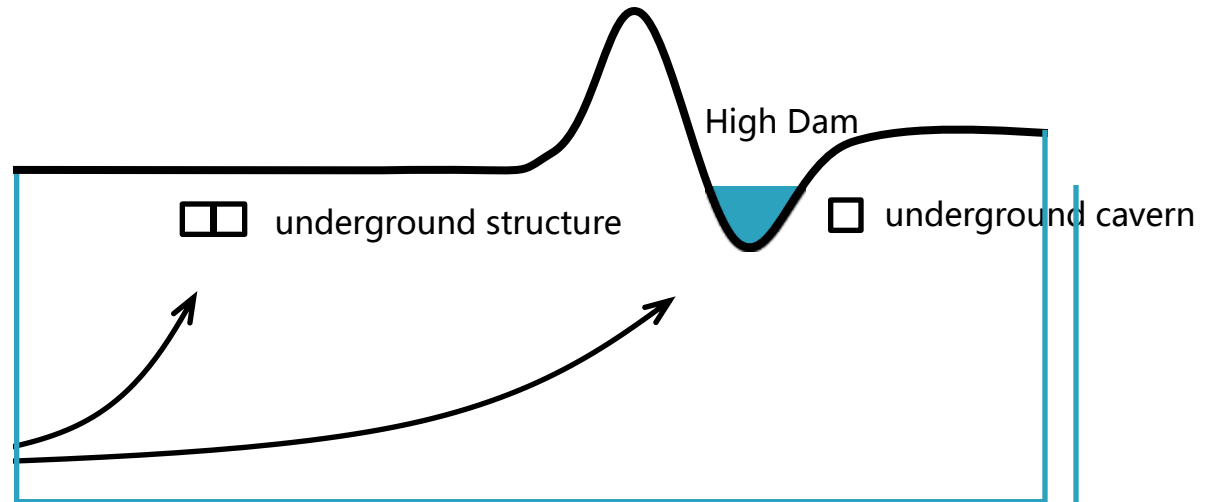
earth-rock dam



underground cavern



urban underground structure

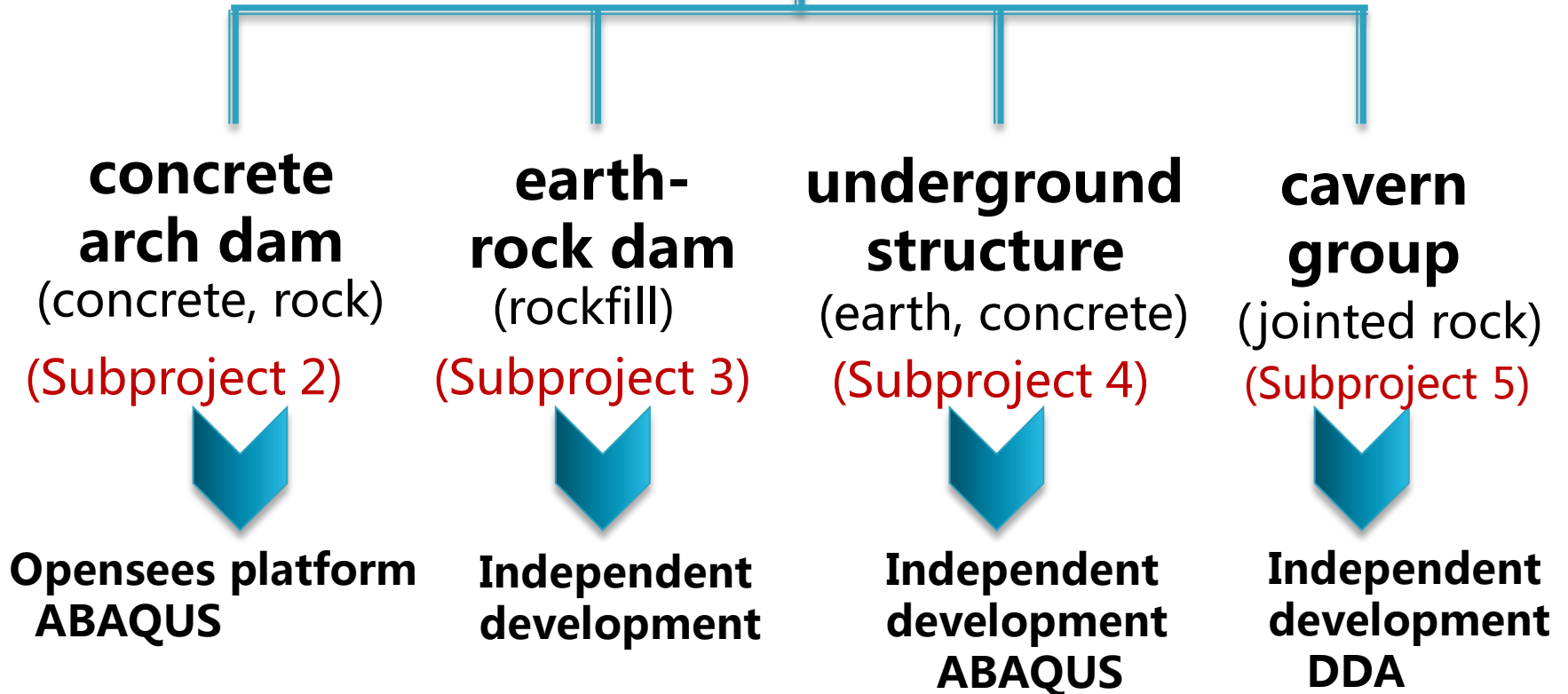


Exogenous problem: seismic input+ artificial boundary
Endogenous problem: hypocenter model+ artificial boundary

II. Overall research methodology

Strong ground motion  Independent development

(Subproject 1)



5 sets of special analysis software (platforms)

Contents

I. Objectives and tasks

II. Overall research methodology

III. Important progress and innovation

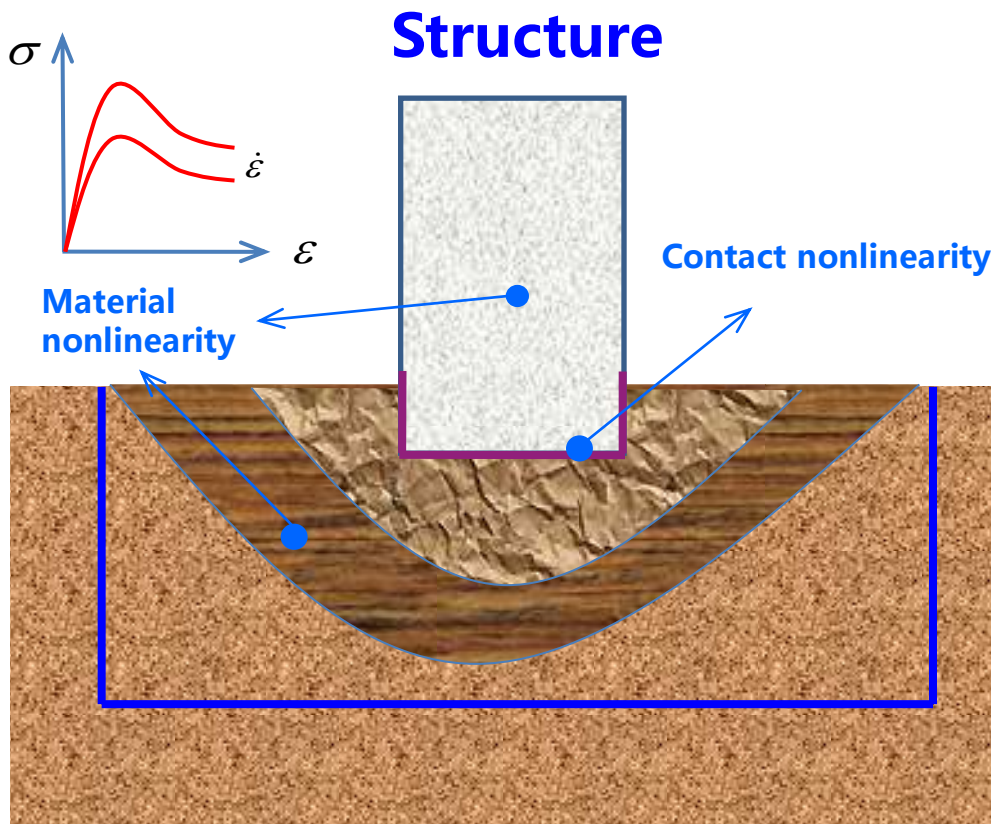
IV. Summary and prospect

III. Important progress and innovation

- 1. Seismic disaster simulation theory and method and system integration**
- 2. Theory and method of broadband seismic field simulation and implementation**
- 3. Seismic failure mode and damage mechanism of key structures**

1 Seismic disaster simulation theory and method and system integration

1.1 Numerical simulation theory and method



Question 1 :

Artificial boundary -finite processing of infinite models

Question 2 :

The establishment of non - linear mechanics model of material and contact

Question 3 :

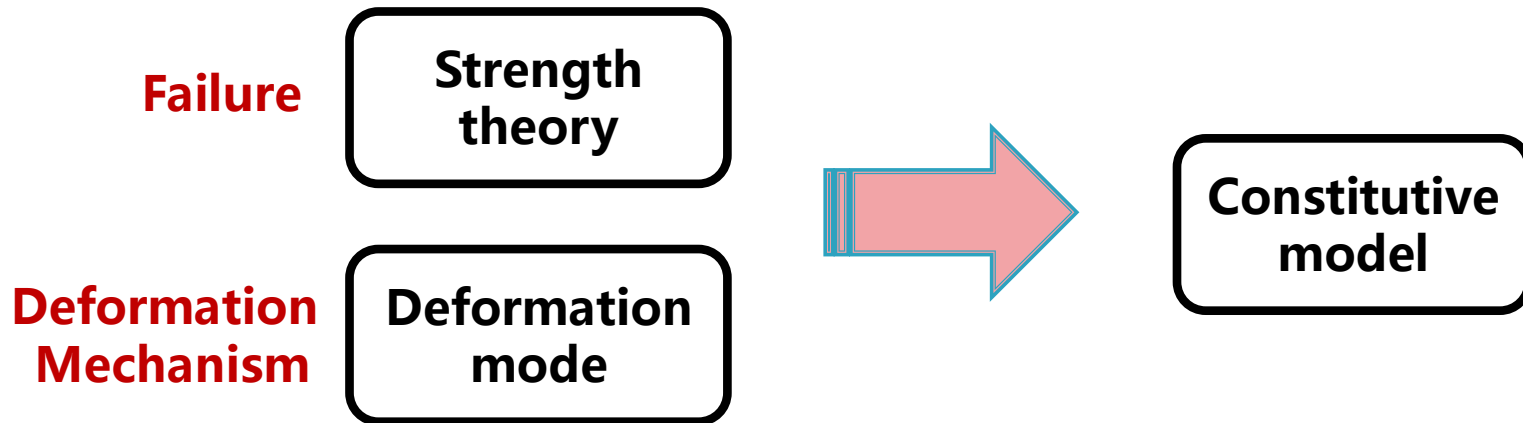
Efficient algorithm for nonlinear fluctuation analysis(coupled) of large - scale complex

1 Seismic disaster simulation theory and method and system integration

□ Dynamic Failure Behavior and Surface Contact Model

(1) Dynamic Failure Behavior Model

Plastic deformation, strain softening, cycle loading, stiffness degradation and etc.

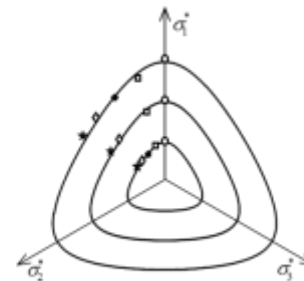
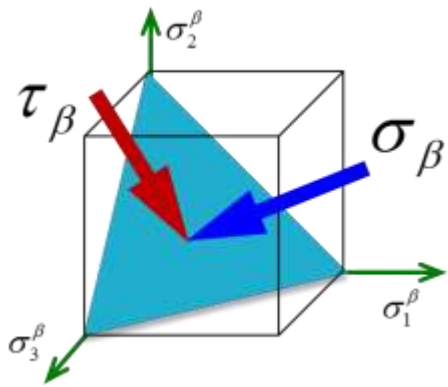


(Damage) constitutive model of geotechnical materials such as overconsolidated soil, rockfill material and concrete

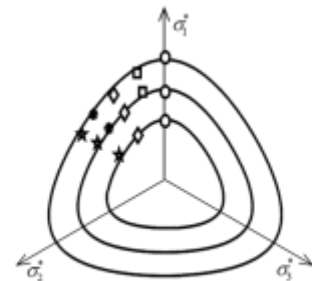
1 Seismic disaster simulation theory and method and system integration

Achievement 1 : Proposed a nonlinear unified strength theory based on the concept of characteristic stress.

Principal stress effect/Shear sliding surface → **Shear sliding surface/Characteristic stress** → **Simple and clear physical definition**



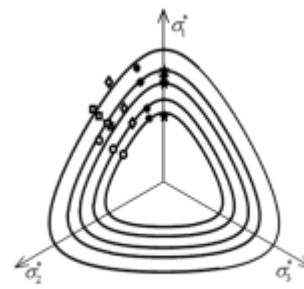
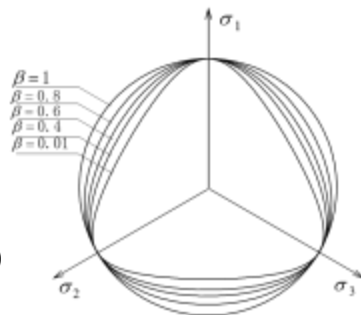
黏土



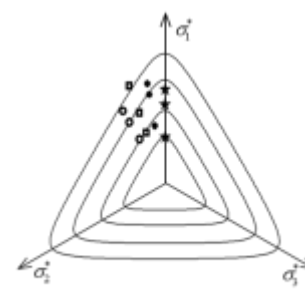
砂土

$$\sigma_i^\beta = p_a \left(\frac{\sigma_i}{p_a} \right)^\beta$$

$$\tau_\beta = M_\beta \sigma_\beta + \sigma_0$$



花岗岩



砂岩



混凝土

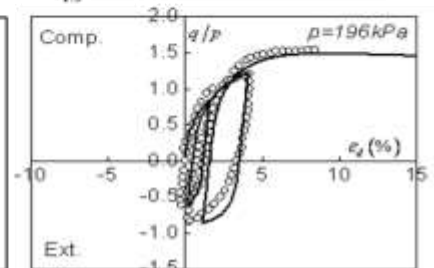
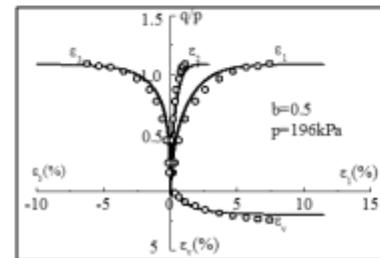
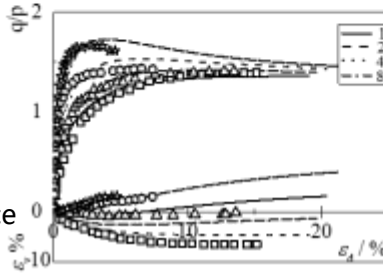
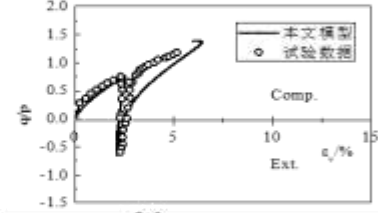
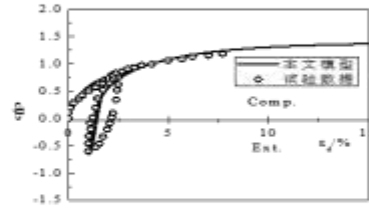
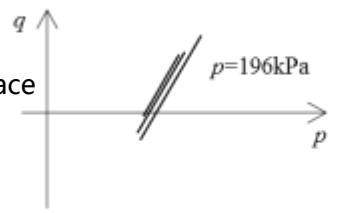
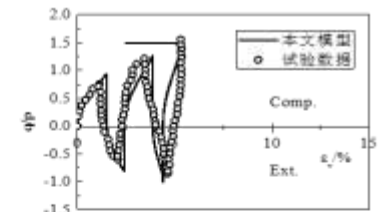
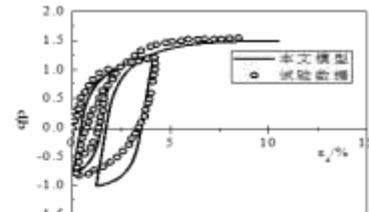
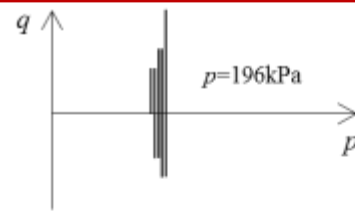
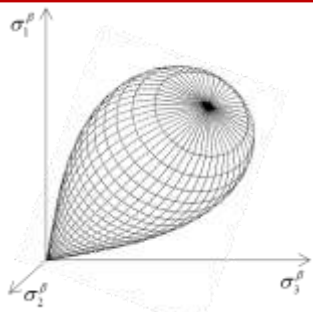
1 Seismic disaster simulation theory and method and system integration

Achievement 2 : Established elasto-plastic (damage) constitutive model of geo-materials for different deformation characteristics

1) Proposed an elastoplastic constitutive model of overconsolidated soil considering strain softening and cyclic loading.

(Less parameters/5, clear physical definition, True 3D, Dilatancy properties, Strain softening, Cyclic loading)

3-axis compression + Three-dimensional method → Direct true 3-D elasto-plastic model → Simple, stress induced heterosexual



The Yield Surface in Characteristic Stress Space

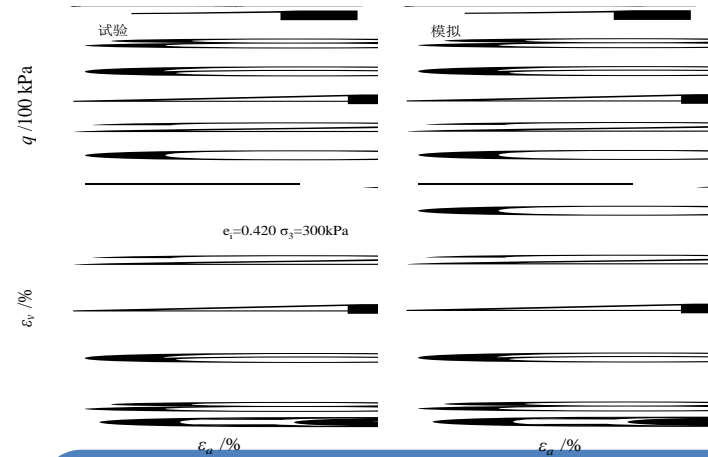
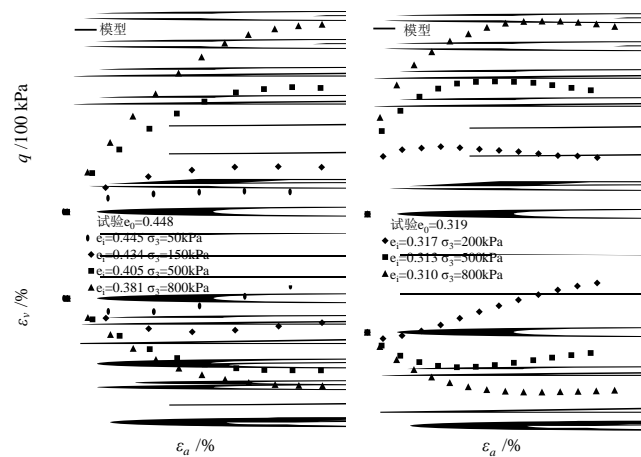
The yield surface in principal stress space

1. Seismic disaster simulation theory and method and system integration

Achievement 2 : Established elasto-plastic (damage) constitutive model of geo-materials for different deformation characteristics

2) Established elasto-plastic constitutive model of rockfill considering the particle break and dilatancy.

(Cyclic loading, particle breakage)



- ◆ Many Models, and poor correlation of parameters
- ◆ Cannot respond to residual deformation time-course changes
- ◆ Cannot reflect the impact of particle fragmentation
- ◆ Cannot reflect state dependencies



- ◆ Monotonic and cyclic use of a set of parameters
- ◆ Consider dilatancy and stress softening
- ◆ Consider complex loading and stress history
- ◆ Consider the effect of particle breakage
- ◆ Reflecting the state of relevance

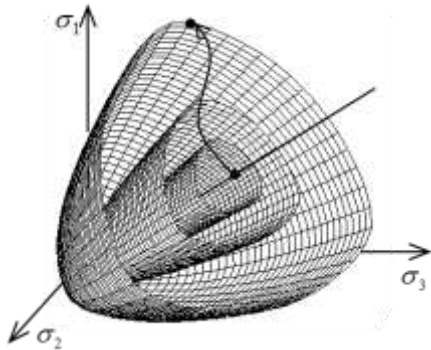
1. Seismic disaster simulation theory and method and system integration

Achievement 2 : The elasto-plastic (damage) constitutive model of geomaterials is established for different deformation characteristics

3) Established elasto-plastic damage constitutive model of concrete considering strain softening, stiffness degradation and cyclic loading.

Single strength criterion → Generalized nonlinear strength theory → Elasto-plastic, damage

$$D = \frac{1}{1 + \left(\frac{1}{D_0} - 1\right) \exp(-\alpha(\varepsilon_p - \varepsilon_{p0})^c)}$$

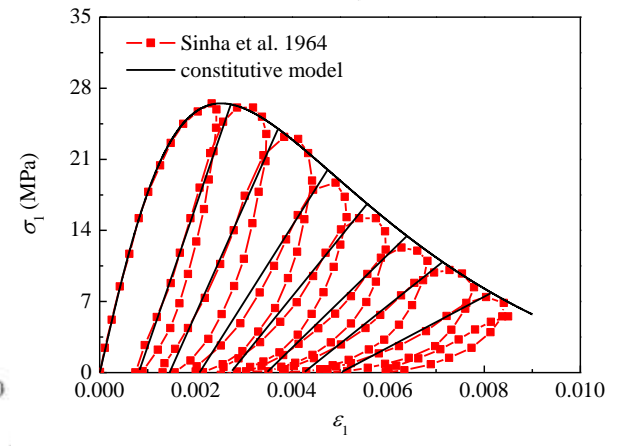
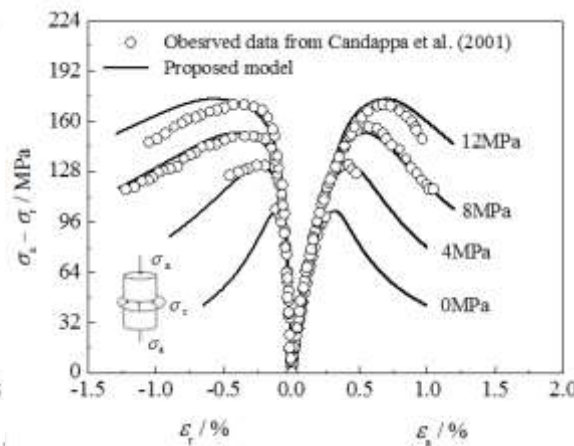
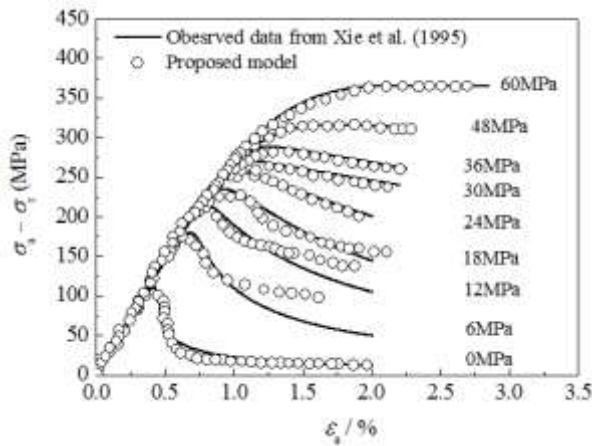
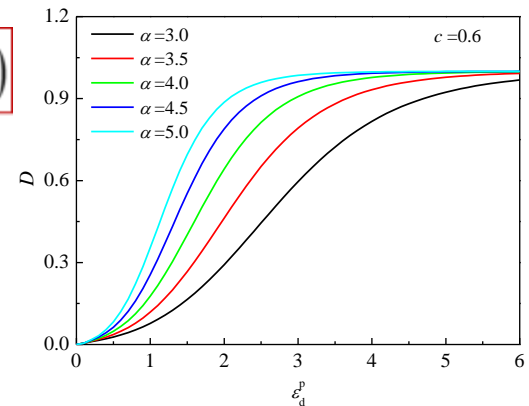


$$\sqrt{(\bar{\sigma}_1^\beta - \bar{\sigma}_2^\beta)^2 + (\bar{\sigma}_2^\beta - \bar{\sigma}_3^\beta)^2 + (\bar{\sigma}_3^\beta - \bar{\sigma}_1^\beta)^2} = H(\varepsilon_d^p)$$

$$x = \varepsilon_d / \varepsilon_{d0} \quad y = q / q_g$$

$$x \leq 1 \quad y = \frac{x}{\beta x + 1 - \beta} - (1 - \beta)(x - x^2)$$

$$x > 1 \quad y = \frac{x}{\gamma(x - 1)^2 + x}$$



1. Seismic disaster simulation theory and method and system integration

Achievement 2 : The elasto-plastic (damage) constitutive model of geomaterials is established for different deformation characteristics

4) Established elasto-plastic damage constitutive model of rock considering strain softening, stiffness degradation and cyclic loading.

New model (cyclic loading, permanent deformation)

Cyclic shear test dilatancy curve of Joint

Experimental expression of average dilution

Relation between average dilatancy angle shear rate and other physical quantities

Ladanyi-Archambault Shear strength model

Cycling Shear strength model of joint

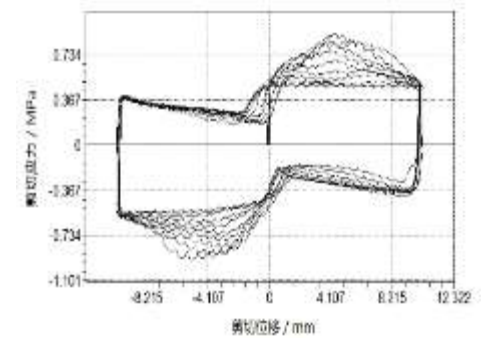
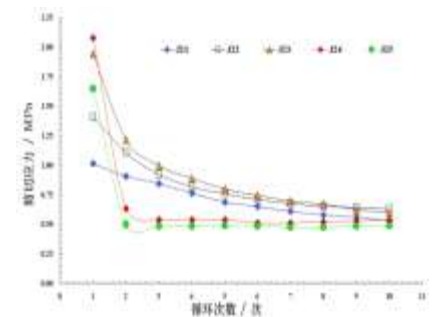
$$\tau_t = \frac{\sigma_n(1-a') \cdot (\tan \alpha_t + \tan \phi) + a' \tau_{rock}}{1 - (1-a') \cdot \tan \alpha_t \cdot \tan \phi}$$

$$\phi_t = \frac{\alpha_t \left(\phi_0 - a \operatorname{ctg} \frac{\sigma_n}{\tau_r} + \alpha_r \right) + \alpha_0 \left(a \operatorname{ctg} \frac{\sigma_n}{\tau_r} - \alpha_r \right) - \phi_0 \alpha_r}{\alpha_0 - \alpha_r}$$

$$\tau_{rock} = \sigma_c \frac{\sqrt{n-1}-1}{n} \sqrt{1+n \frac{\sigma_n}{\sigma_c}}$$

$$\alpha_t = \alpha_0 \cdot R_s = \alpha_0 \cdot \left[A \left(e^{\frac{1}{n}} - 1 \right) + 1 \right]$$

$$\alpha'_t = \frac{\left[(2H - L \cdot \tan \alpha_t)^2 \cdot \left(\sin \alpha_t + \frac{\cos \alpha_t}{\operatorname{tg}(\alpha_0 - \alpha_t)} \right) - (2H - L \cdot \tan \alpha_{t-1})^2 \cdot \left(\sin \alpha_t + \frac{\cos \alpha_t}{\operatorname{tg}(\alpha_0 - \alpha_{t-1})} \right) \right] \cos \alpha_t}{16S_0}$$



1. Seismic disaster simulation theory and method and system integration

Dynamic Failure Behavior and Interface Contact Model

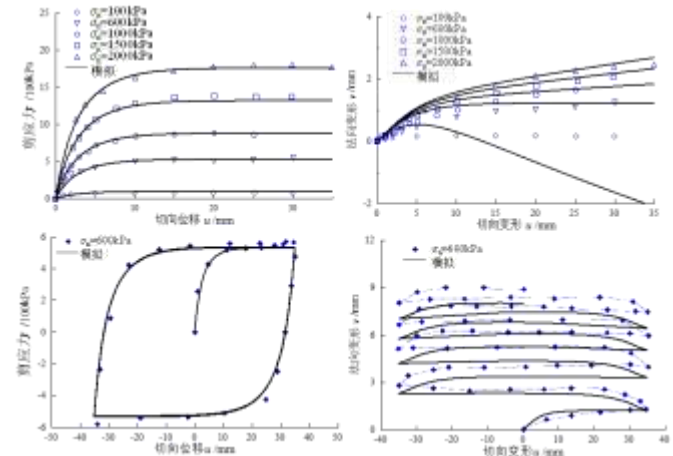
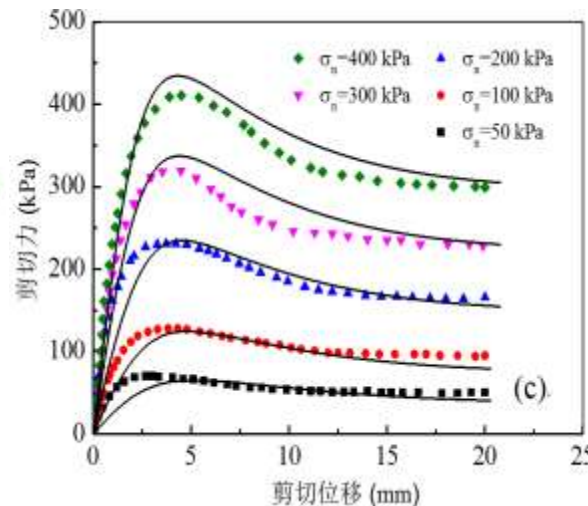
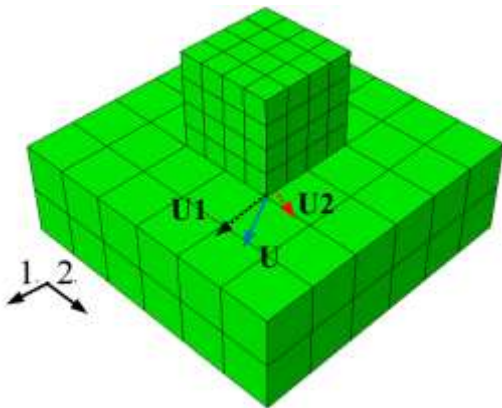
(2) Interface model between soil and structure

Interface element : Goodman element Desai element Shear stiffness ?

Solution: taking usage of the relationship between the shear stress and the shear strain on the sliding surface of the constitutive relationship of soil

Achievement 1 : Based on the constitutive model of overconsolidated soil, the contact shear stiffness model was established

Achievement 2 : Based on plasticity constitutive model of rockfill material, the model of interface contact shear stiffness was established

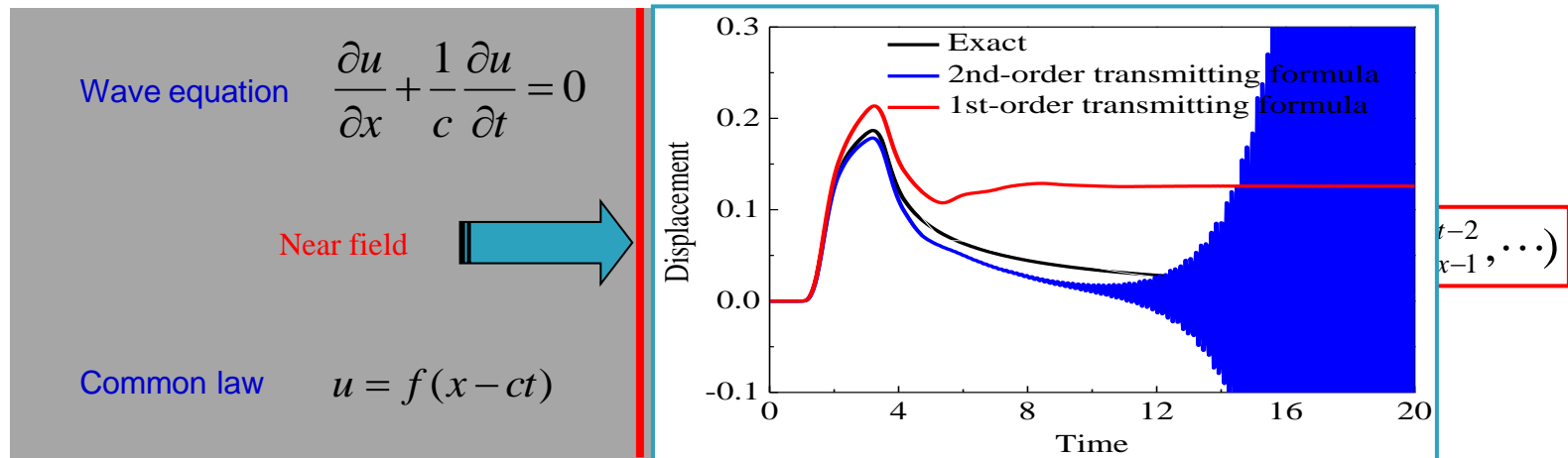


1. Seismic disaster simulation theory and method and system integration

□ Artificial Boundary and Input of Ground Motion

(1) Artificial Boundary

Major achievements of international studies in the last 4 decades
---- Displacement Artificial Boundary Condition



Artificial boundary

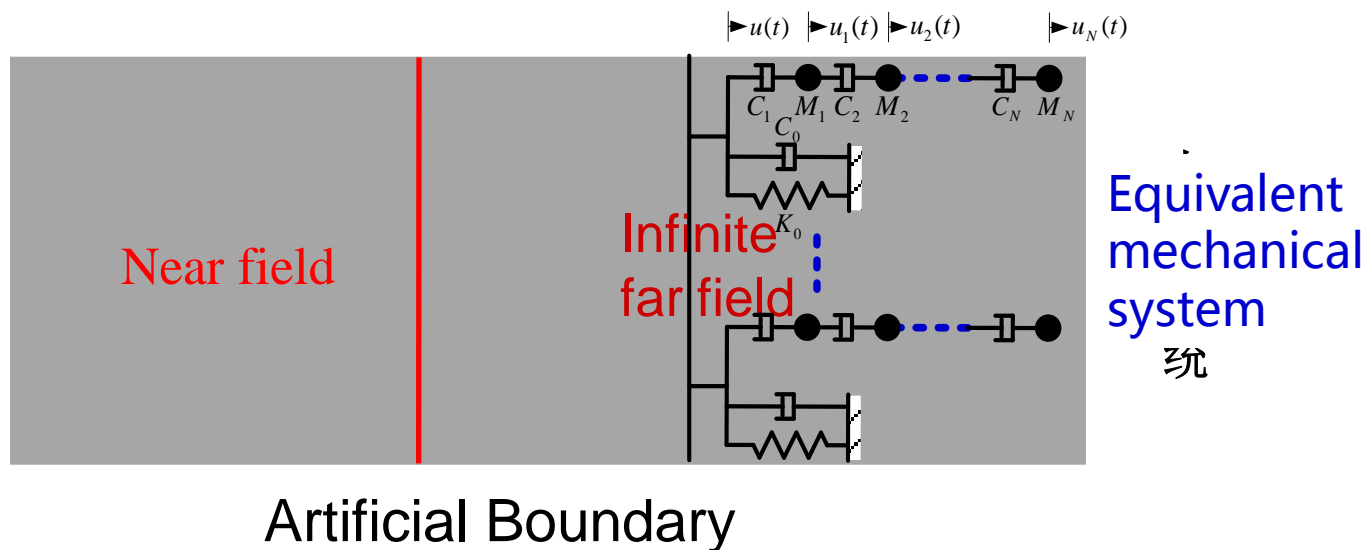
- ① Low-frequency drift and high-frequency oscillation instability phenomenon
- ② Inconvenience in the commercial finite element software

1. Seismic disaster simulation theory and method and system integration

□ Artificial Boundary and Input of Ground Motion

(1) Artificial Boundary

Important development direction ---- High Order Precision Stress Artificial Boundary Condition



- ① The equivalent mechanical system is consistent with the finite element equation
- ② To avoid low-frequency drift and high-frequency oscillation instability

1. Seismic disaster simulation theory and method and system integration

Achievement 1 : A Systematic Method for Constructing High - Precision Time Domain Artificial boundary is proposed

Boundary force-Displacement relationship(Frequency domain)

Scalar $F(\omega) = S(\omega)U(\omega)$

Vector $\mathbf{F}(\omega) = \mathbf{S}(\omega)\mathbf{u}(\omega)$

Force-displacement relationship Approach to the Stability of a High Order Linear System

$$S(\omega) \approx \frac{p_0 + p_1 i\omega + \dots + p_{N+1} (i\omega)^{N+1}}{q_0 + q_1 i\omega + \dots + q_N (i\omega)^N}$$

$$\mathbf{S}(\omega) = \mathbf{g}_0 + i\omega\mathbf{h}_0 - i\omega\mathbf{S}_1^{-1}$$

$$\mathbf{S}_j = \mathbf{g}_j + i\omega\mathbf{h}_j - i\omega\mathbf{S}_{j+1}^{-1}$$

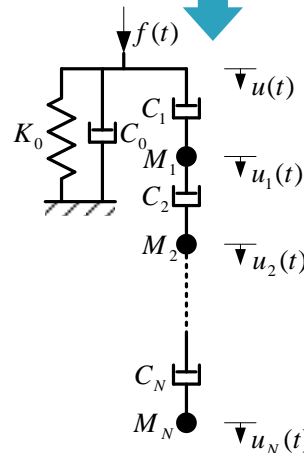
$$j = 1, \dots, N$$

Rational function

Continuous fraction

equivalence

Containing internal variables (reduced order) Time - domain state space system



$$\begin{bmatrix} \mathbf{C}_{bb} & \mathbf{C}_{bc} \\ \mathbf{C}_{cb} & \mathbf{C}_{cc} \end{bmatrix} \begin{Bmatrix} \dot{\mathbf{u}} \\ \dot{\bar{\mathbf{u}}} \end{Bmatrix} + \begin{bmatrix} \mathbf{K}_{bb} & \mathbf{K}_{bc} \\ \mathbf{K}_{cb} & \mathbf{K}_{cc} \end{bmatrix} \begin{Bmatrix} \mathbf{u} \\ \bar{\mathbf{u}} \end{Bmatrix} = \begin{Bmatrix} \mathbf{f} \\ \mathbf{0} \end{Bmatrix}$$

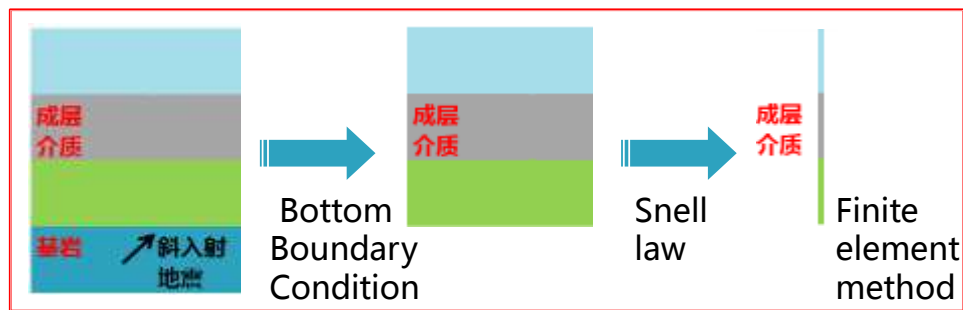
1. Seismic disaster simulation theory and method and system integration

□ Artificial Boundary and Input of Ground Motion

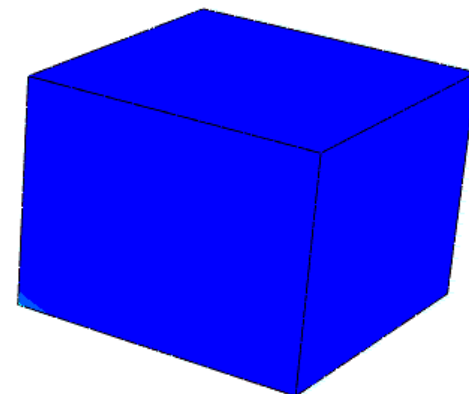
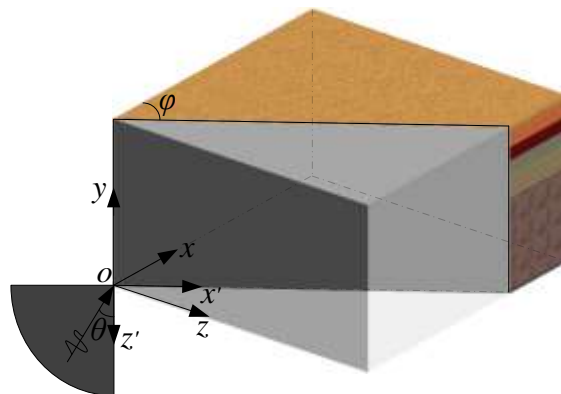
(2) Ground motion input

Results: established seismic oblique incidence input method for stratified half-space field

Algorithm to achieve:



Calculation model
With results :

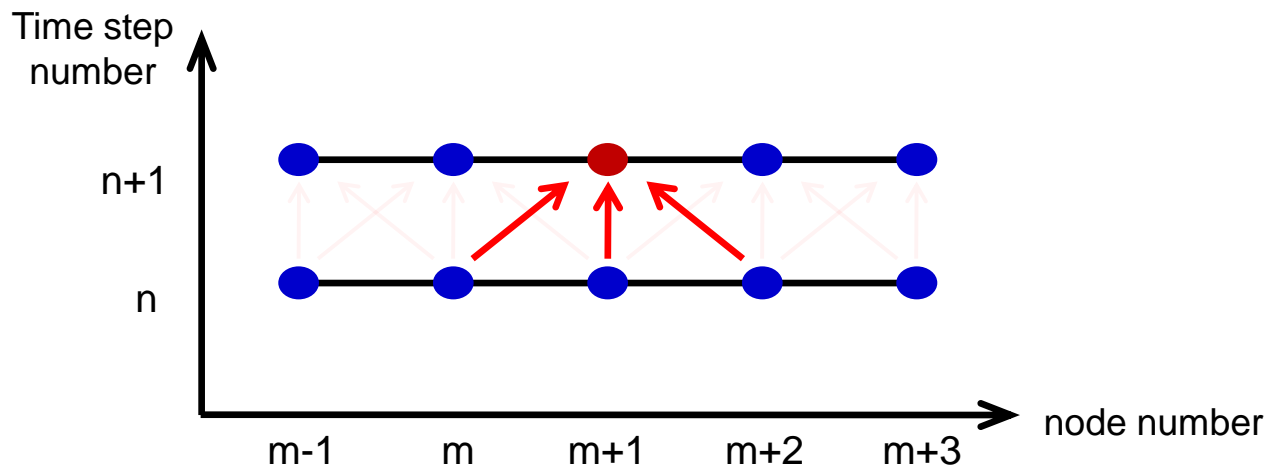


1. Seismic disaster simulation theory and method and system integration

□ Efficient and high precision numerical method

(1) Efficient algorithm—Space - time Localization Method

- Based on the concept of finite velocity, the current time response of a node is only related to the reaction of the first few moments of the neighboring nodes.
- It is not necessary to solve the coupled equations, and the computational efficiency is high, saving memory and being suitable for parallel computation.



a) **Differential equation--Finite difference** Space-time decoupling

b) **Integral equation--Finite element method** Necessary Conditions for Space - Time Decoupling Time domain difference Centralized quality model

1. Seismic disaster simulation theory and method and system integration

□ Efficient and high precision numerical method

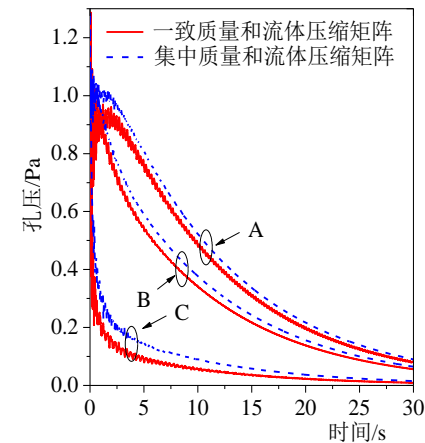
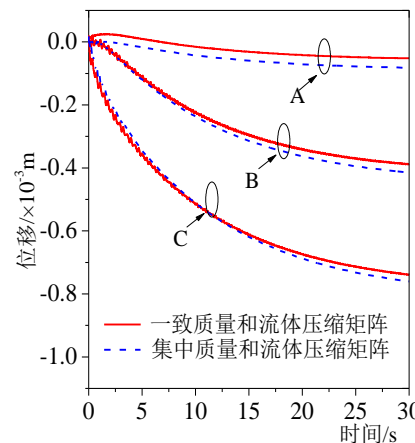
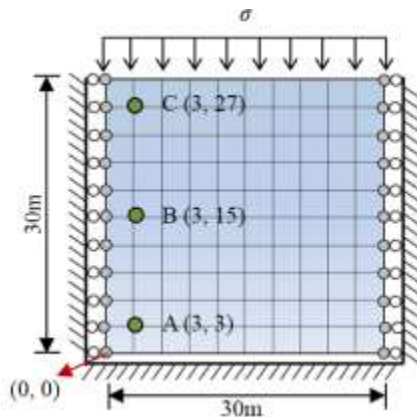
Achievement 1 : The explicit finite element method of viscoelastic solid medium of D'Alembert was developed

Achievement 2 : The explicit finite element method of saturated two-phase media is developed

Achievement 3 : An explicit algorithm for the dynamic contact force and the dynamic contact force of the solid medium is established

$$u_{k+1} = M^{-1} (f_{uk} - K_d \cdot u_k + Q \cdot p_k) \cdot \Delta t^2 + 2 \cdot u_k - u_{k-1}$$
$$p_{k+1} = T \cdot p_k + \frac{\Delta t}{2} \sum_{i=1}^n \omega_i \cdot T \left(\frac{\Delta t}{2} (1 - \zeta_i) \right) \cdot S^{-1} \cdot f_q \left[t_k + \frac{\Delta t}{2} (1 + \zeta_i) \right]$$
$$+ \frac{\Delta t}{2} \left[\frac{S^{-1} Q^T}{\Delta t} u(t_{k+1}) + \frac{S^{-1} Q^T (T - 1)}{\Delta t} u(t_k) - \frac{TS^{-1} Q^T}{\Delta t} u(t_{k-1}) \right]$$

Quality matrix M
Fluid compression matrix S
Using a unified diagonal processing



1. Seismic disaster simulation theory and method and system integration

□ Efficient and high precision numerical method

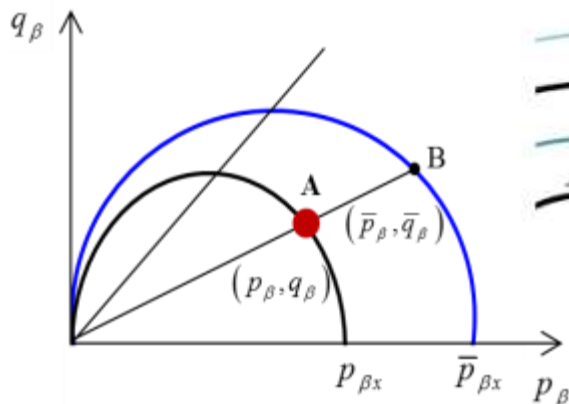
(1) Efficient algorithm—Elastic - Plastic Stress Updating Algorithm

Existing stress update algorithm : Elastoplastic iteration (Increment)+ Nonlinear iteration → **high calculation cost, low precision**

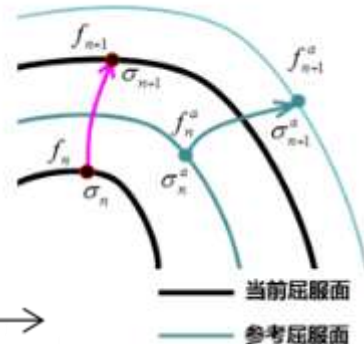
Proposed stress updating algorithm: loading and unloading judgment+ Nonlinear iteration → **Small calculation amount, high precision**

Strain hardening + Strain softening = Judged by the over-consolidation ratio

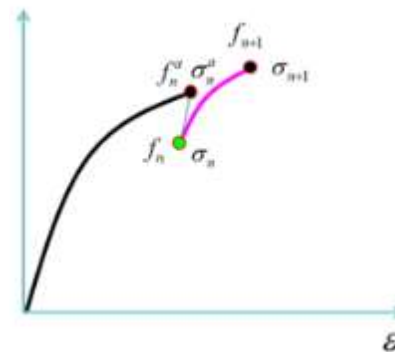
Suitable for large-scale computing



3- Dimensional Elastic - Plastic Model of Lower Loading Surface



Elastic - Plastic Stress Updating Algorithm



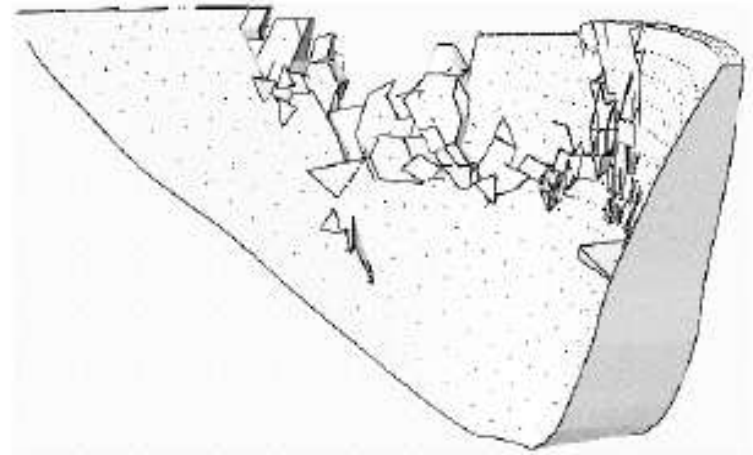
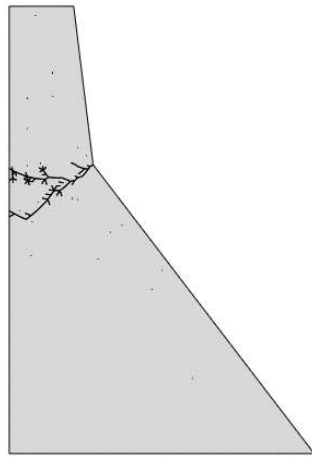
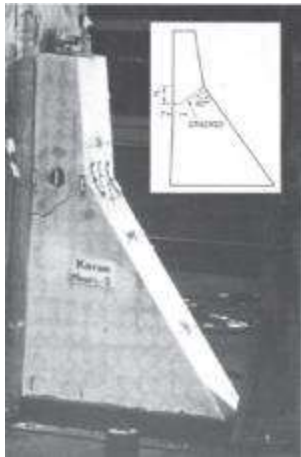
Loading and unloading guidelines

1. Seismic disaster simulation theory and method and system integration

□ Efficient and high precision numerical method

(2) **Grid Dependency**—Strain softening, cracking and stress concentration etc.

Achievement : Apply extended finite element model and thickness-free bonding element model to the analysis of high dam



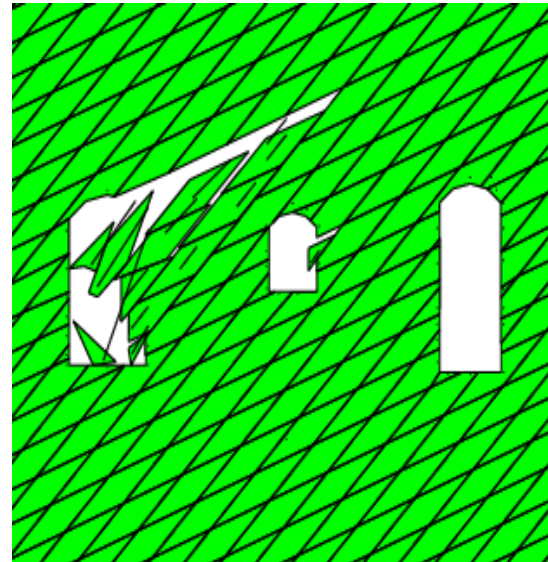
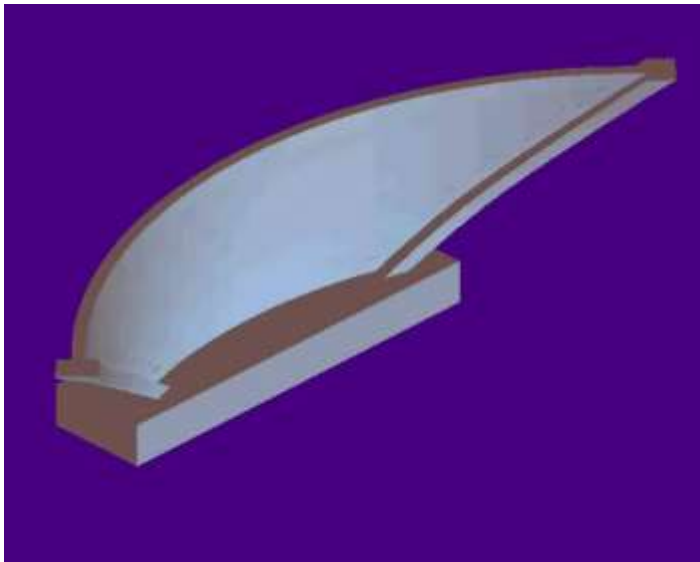
1. Seismic disaster simulation theory and method and system integration

□ Efficient and high precision numerical method

(3) Discontinuous deformation problem

DDA, Discrete element and Manifold etc.

Achievement : Apply DDA and discrete element method to seismic hazard analysis of high dam and underground cavern




1. Seismic disaster simulation theory and method and system integration

1.2 Physical Simulation Theory and Methodology

◆ test means-shaking table (1-G,N-G)


defect : Uniform similarity can not be satisfied



1-G

advantage : Physical Modeling of Large - Scale Target Structures

defect : Small size and Coriolis effect



N-G

advantage : meet the stress similar

1. Multi - media coupling similarity ; 2. Weakening Design of Model Materials ; 3. Selection of ground motion

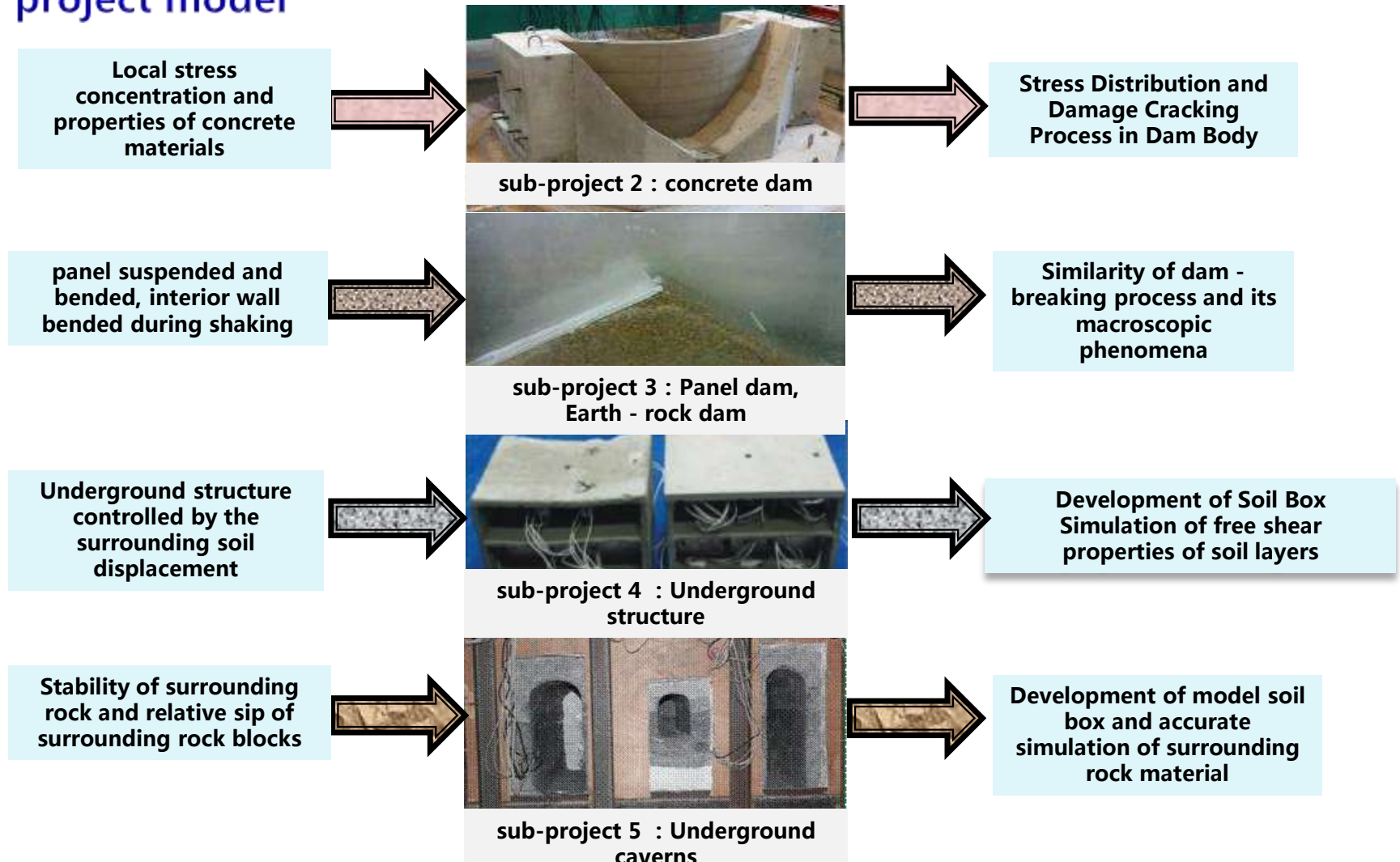
1、 semi qualitative - quantitative analysis。

2、 Validation of mature numerical algorithms , to provide technology support for the simulation sets.

1. Seismic disaster simulation theory and method and system integration

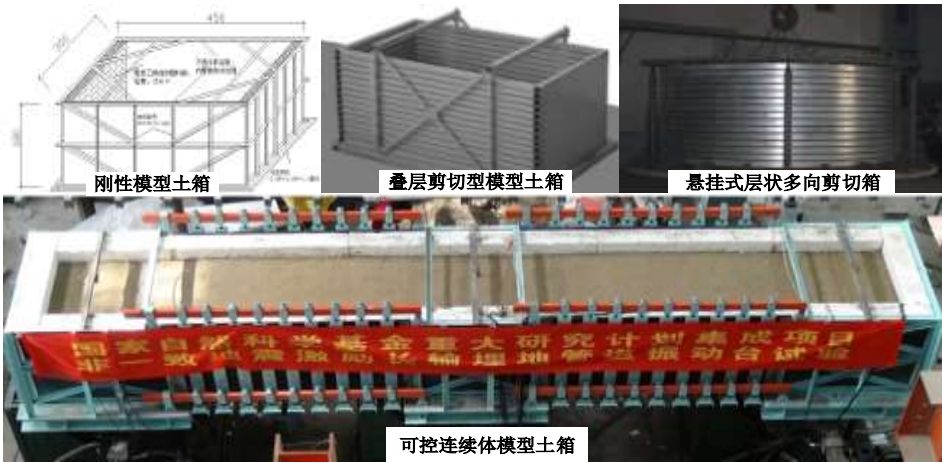
1.2 Physical Simulation Theory and Methodology

- ◆ The structural damage characteristics and test focus of each sub-project model

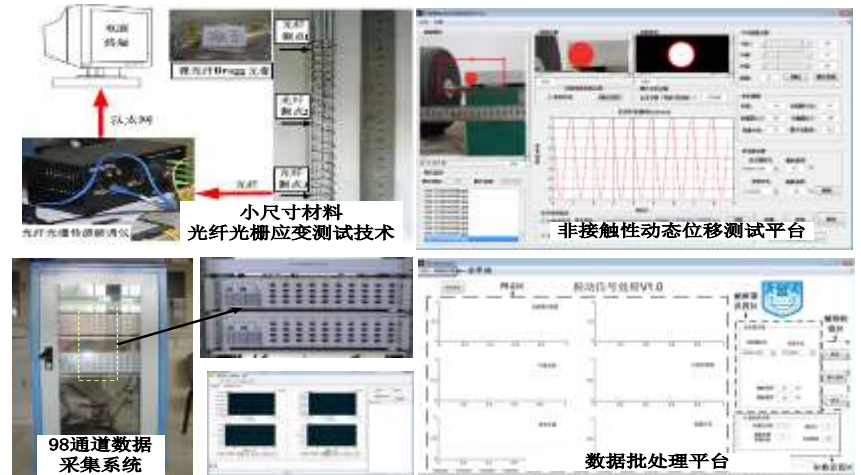


1. Seismic disaster simulation theory and method and system integration

Achievement 1 : New model box design, test technology, measurement and data transmission technology



Subproject 4



Subproject 4



Subproject 2



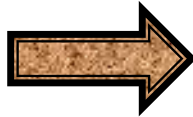
Subproject 3

1. Seismic disaster simulation theory and method and system integration

Achievement 2 : Development of Model Materials



Subproject 2



Water : cement : lime : sand = 0.5:1:0.58:5



Subproject 5: new materials for underground cavern



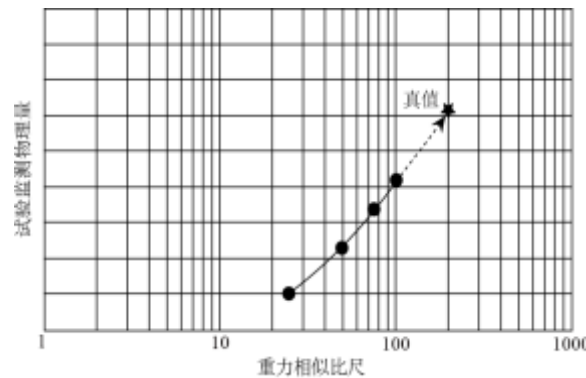
Plaster, Barite powder, Quartz sand, water and Gypsum retarder

1. Seismic disaster simulation theory and method and system integration

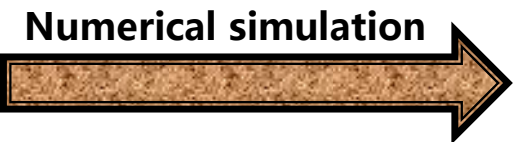
Achievement 3 : Model similarity ratio design

01

A small-scale centrifugal model test method by stress approximation(N-G)



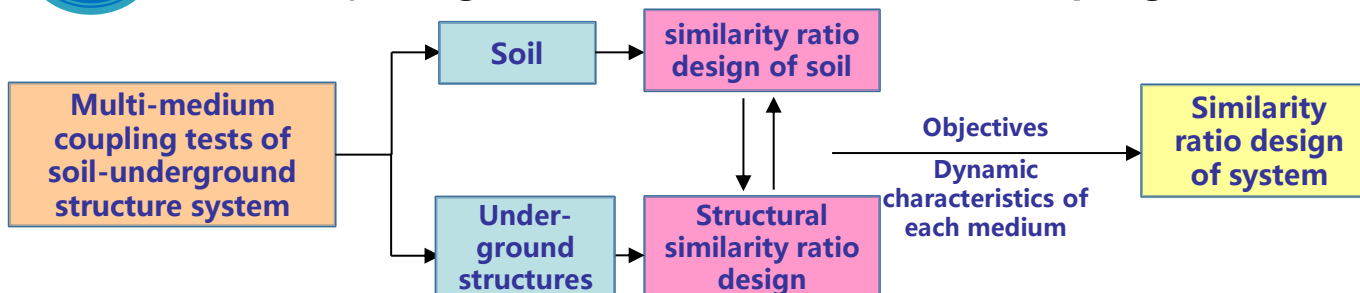
A similar method with good feasibility is obtained by extension analysis by sub project 3



Model similarity ratio design

02

Similarity design method of multi-medium coupling soil-structure system (1-G)



The soil-structure similarity ratio design problem could base on the dynamic characteristics of the foundation soil by Subproject 2

1. Seismic disaster simulation theory and method and system integration

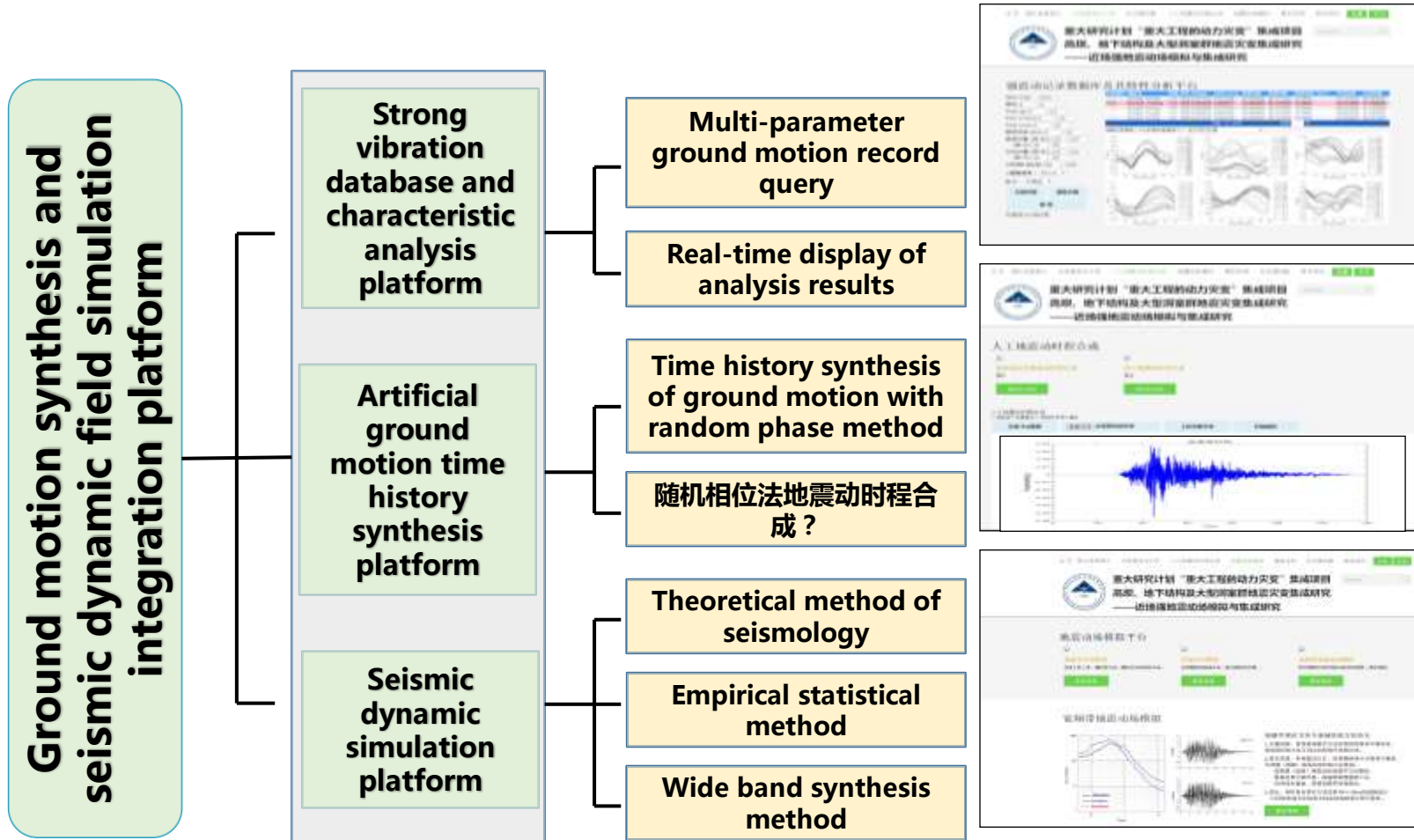
1.3 System integration

- ① Developed a software platform for seismic time history synthesis and seismic field simulation
- ② Developed a high concrete dam seismic analysis software
- ③ Improve and develop the high earth and rock dam seismic simulation software platform (GEODYNA6.0)
- ④ Developed a large-scale underground structure earthquake simulation software (GDA)
- ⑤ Developed a large-scale underground caverns earthquake simulation software



1. Seismic disaster simulation theory and method and system integration

Achievement 1: Seismic Ground Motion Synthesis and Seismic Field Simulation Platform



1. Seismic disaster simulation theory and method and system integration

Achievement 2: high concrete dam seismic analysis software

The analysis software of earthquake disaster of high concrete dam is developed and verified by the model test and the actual earthquake damage based on ABAQUS and OpenSees.

Basic function

Infinite foundation
radiation damping

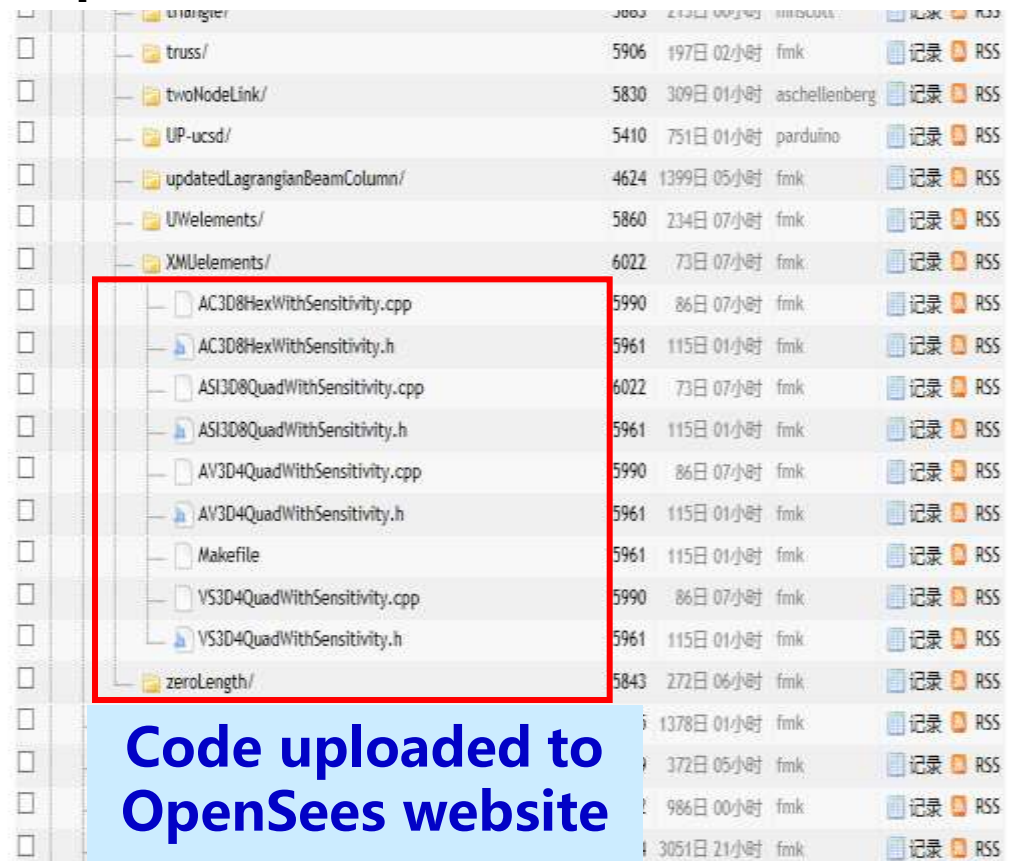
Dam-water interaction

Energy dissipation in
upper reaches of reservoir

Nonlinearity of dam
material

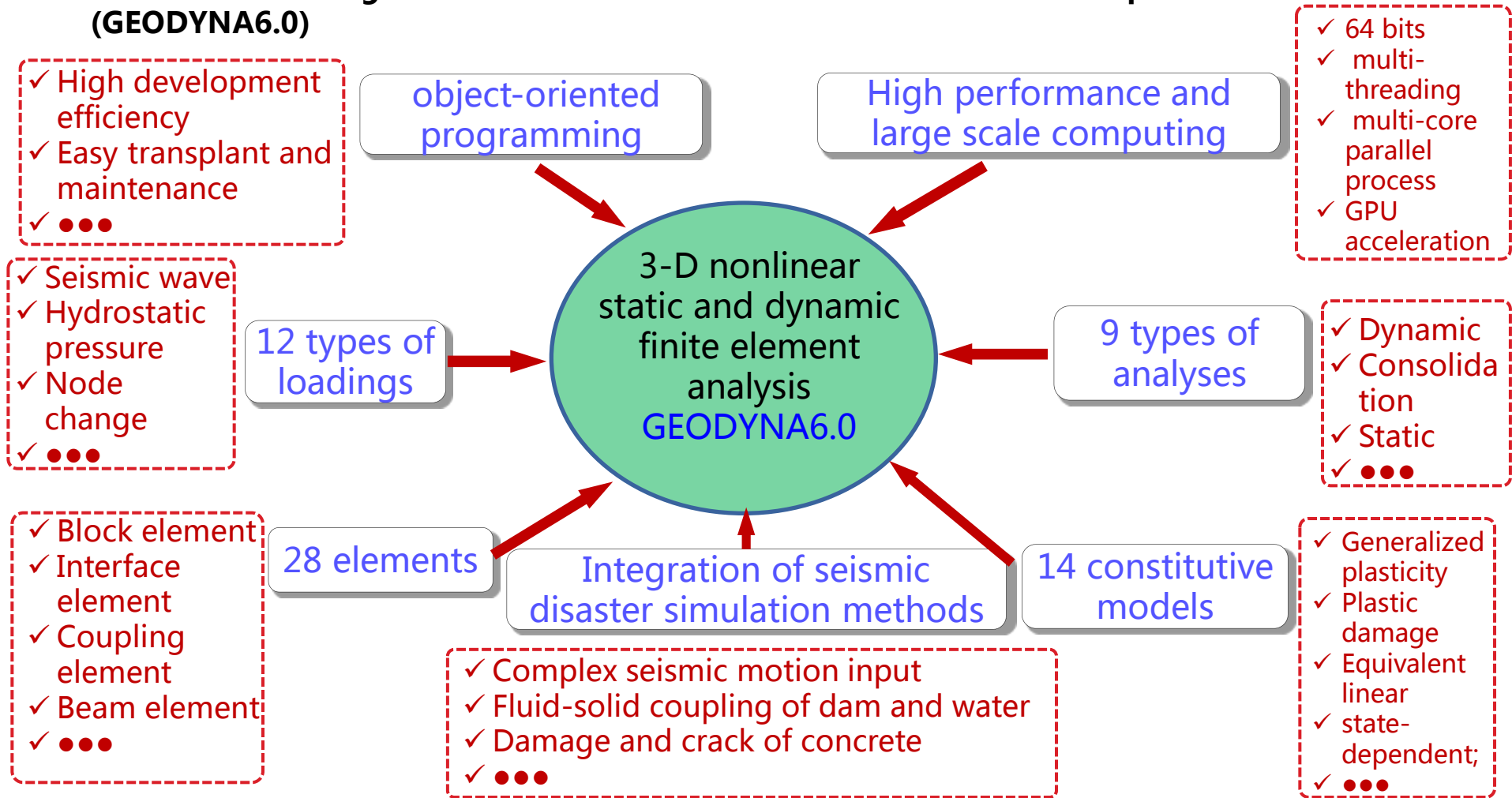
Nonlinearity of transverse
joints of dam

Seismic reinforcement
measures of dam



1. Seismic disaster simulation theory and method and system integration

Achievement 3: High earth and rock dam seismic simulation software platform (GEODYNA6.0)



The calculation scale breaks through 50 million degrees of freedom (static problems) and 10 million degrees of freedom (dynamic problems), is able to solve large-scale, non-linear and earthquake-destroying calculation problems of the 300-meter high earth-rock dams.

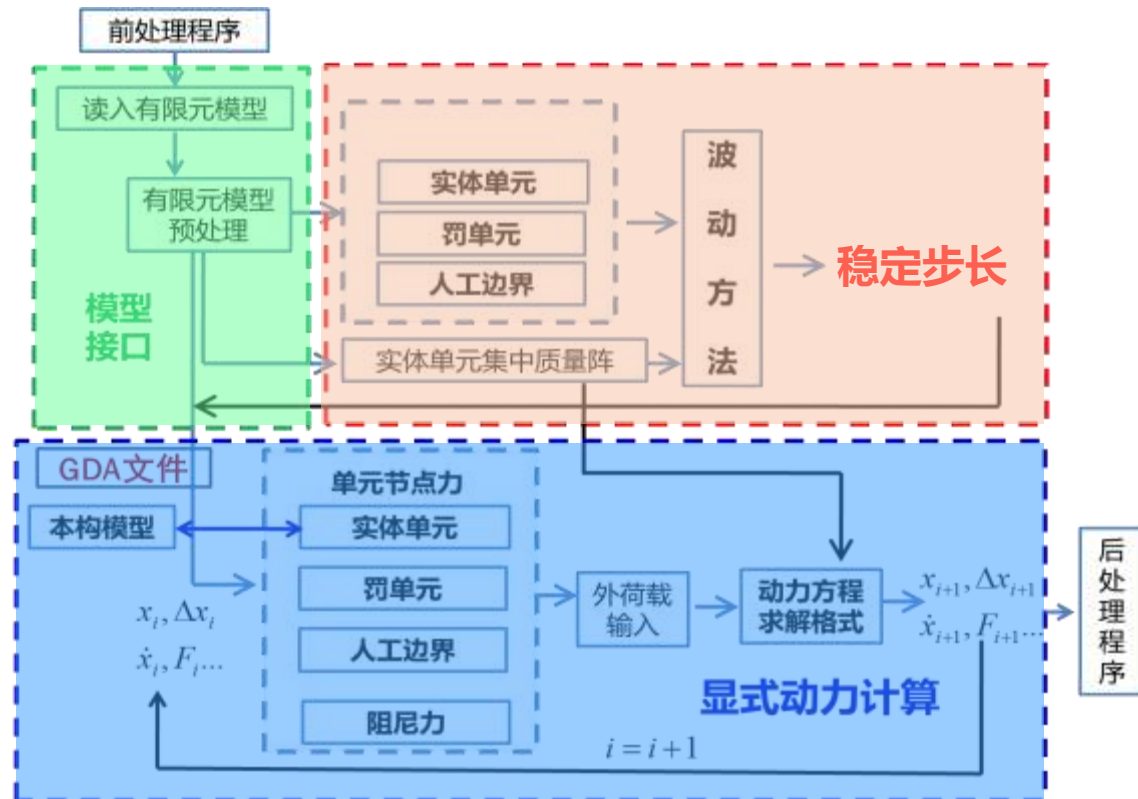
1. Seismic disaster simulation theory and method and system integration

Achievement 3 : Large-scale underground structure earthquake simulation software (GDA)

Geotechnical Dynamic Analysis(GDA) program

Complete independent development of GDA effectively handled the technical difficulties e.g. high computational costs and high nonlinearity during the nonlinear dynamic analyses of underground structures.

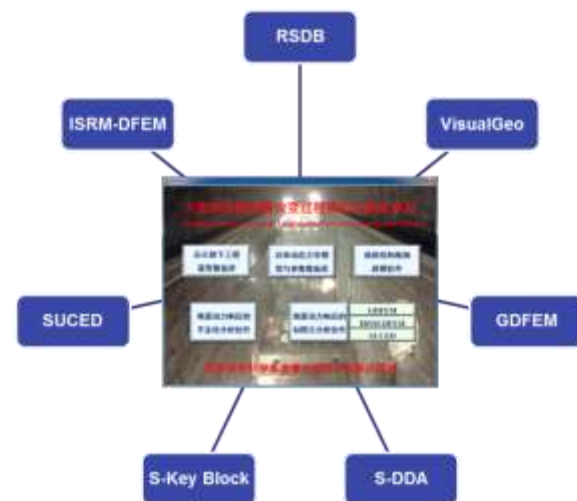
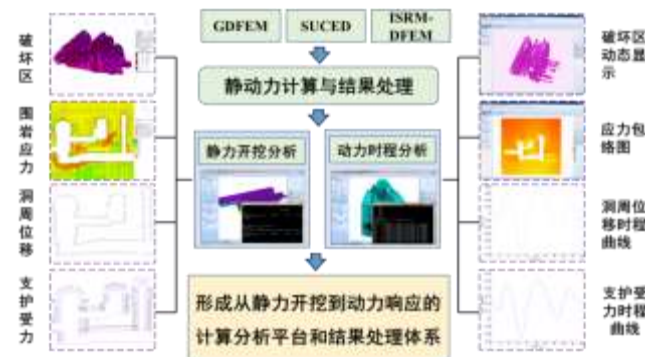
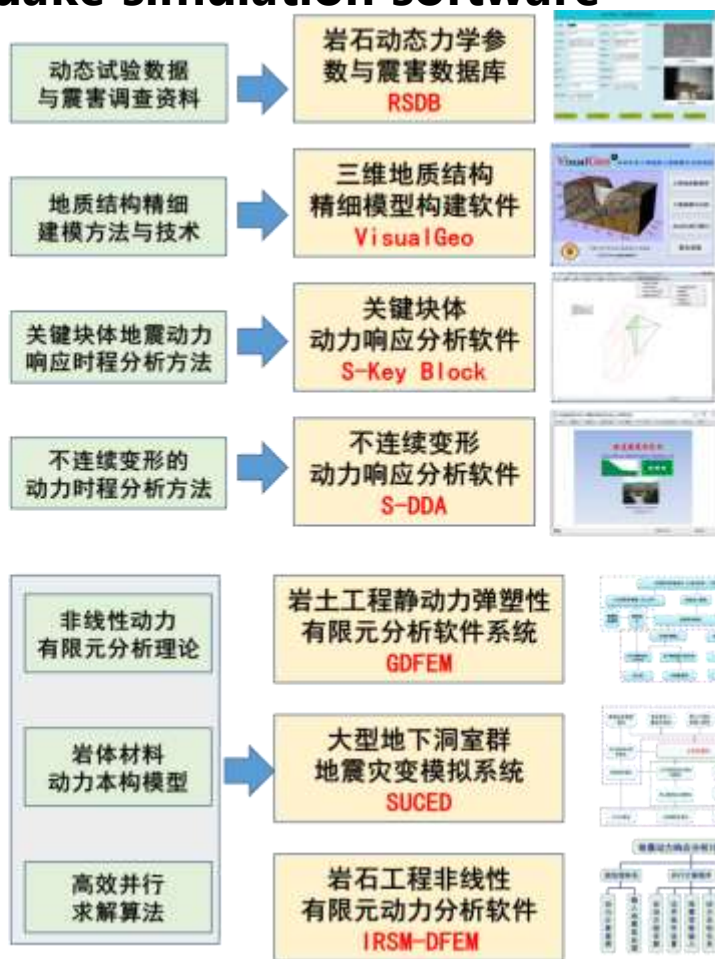
GDA has the characteristics of high scalability, ease of development of new algorithms and models, providing excellent software platform for the integration of research achievements.



Fine and efficient nonlinear finite element analysis of underground structures and software platform for research

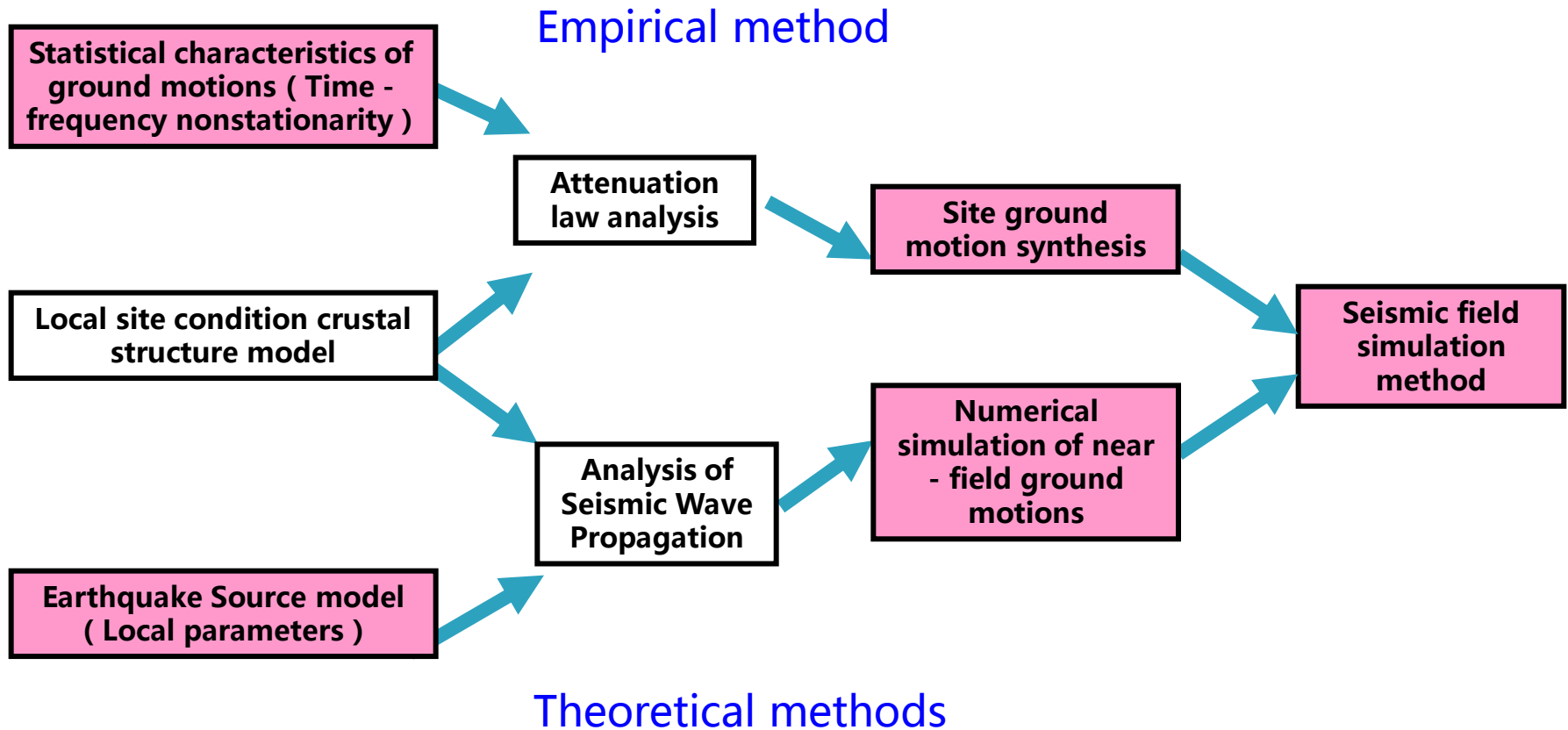
1. Seismic disaster simulation theory and method and system integration

Achievement 5 : Developed a large-scale underground caverns earthquake simulation software



Local and overall combination of efficient, fast, multi-level seismic response and disaster simulation platform

1. Seismic disaster simulation theory and method and system integration



IV. Summary and prospect

- 1. Study on the theories and methods of earthquake disaster simulation.**
- 2. Several progress on physical model tests and a series of destructive tests in this research.**
- 3. New recognitions on the seismic damage mechanism and failure modes.**

Acknowledgement

Thanks !

